

**MiTek Industries, Inc.**

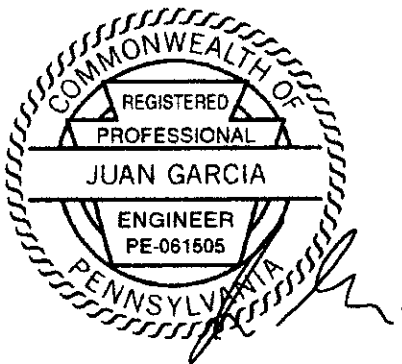
14515 North Outer Forty Drive  
Suite 300  
Chesterfield, MO 63017-5746

Re: G6195  
E.G.Stoltzfus-#71 Barons Ridge

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by J.C. Snavely.

Pages or sheets covered by this seal: I11200484 thru I11200492

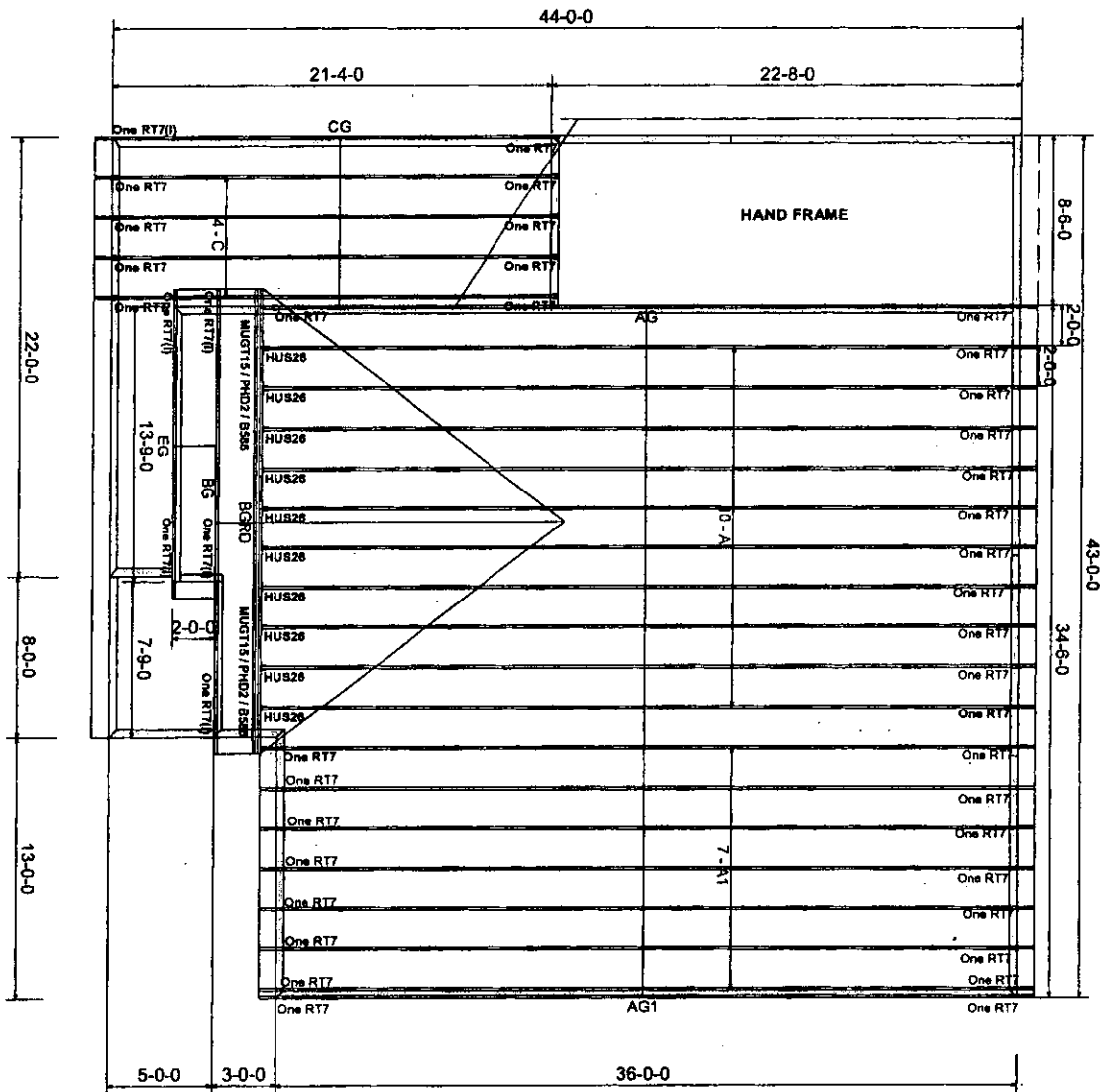
My license renewal date for the state of Pennsylvania is September 30, 2007.



November 2, 2006

Garcia, Juan

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.



E.G. STOLTZFUS  
 #71 BARONS RIDGE - CARLTON MODEL  
 G6195  
 10-25-2006

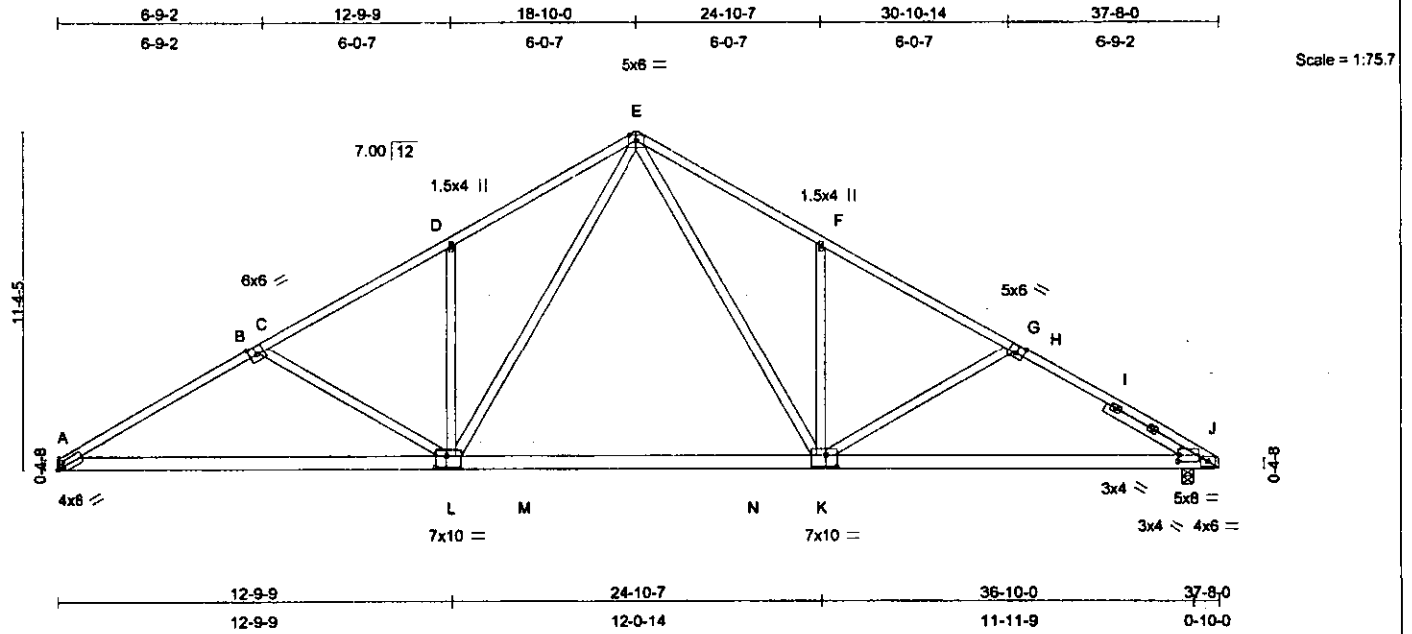


Plate Offsets (X,Y): [A:0-2-12,0-2-0], [B:0-3-0,Edge], [H:0-3-0,0-3-4], [J:0-0-8,0-2-8], [K:0-4-4,0-4-8], [L:0-4-4,0-4-8]

<b>LOADING (psf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plates Increase 1.15	TC 0.56	Plates Vert(LL) -0.42 K-L >999	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.93	Vert(TL) -0.61 K-L >737		
BCLL 0.0	Rep Stress Incr YES	WB 0.93	Horz(TL) 0.10 J n/a		
BCDL 10.0	Code IRC2003/TPI2002	(Matrix)			Weight: 236 lb

**LUMBER**

TOP CHORD 2 X 4 SYP No.2  
 BOT CHORD 2 X 6 SYP No.2  
 WEBS 2 X 4 SYP No.3  
 SLIDER Right 2 X 4 SYP No.3 3-4-15

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 2-9-8 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

**REACTIONS (lb/size)**

A=2019/Mechanical, J=2020/0-4-8  
 Max Horz A=335(load case 5)  
 Max Uplift A=-307(load case 6), J=-306(load case 7)

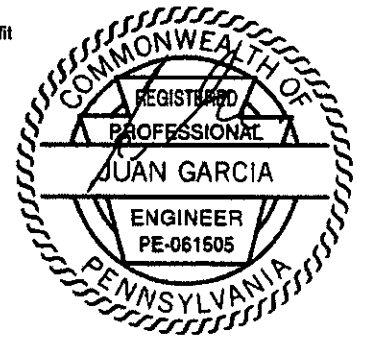
**FORCES (lb) - Maximum Compression/Maximum Tension**

TOP CHORD A-B=-3478/631, B-C=-3214/633, C-D=-2978/545, D-E=-2965/700, E-F=-2955/701, F-G=-2963/544, G-H=-3193/626,  
 H-I=-3315/626, I-J=-3429/597  
 BOT CHORD A-L=-517/2888, L-M=-123/1804, M-N=-123/1804, K-N=-123/1804, J-K=-447/2869  
 WEBS D-L=-493/273, F-K=-503/277, C-L=-504/272, E-L=-337/1390, E-K=-338/1373, G-K=-493/263

**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 90mph; h=22ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint A.
- 7) One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at jt(s) J.
- 8) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



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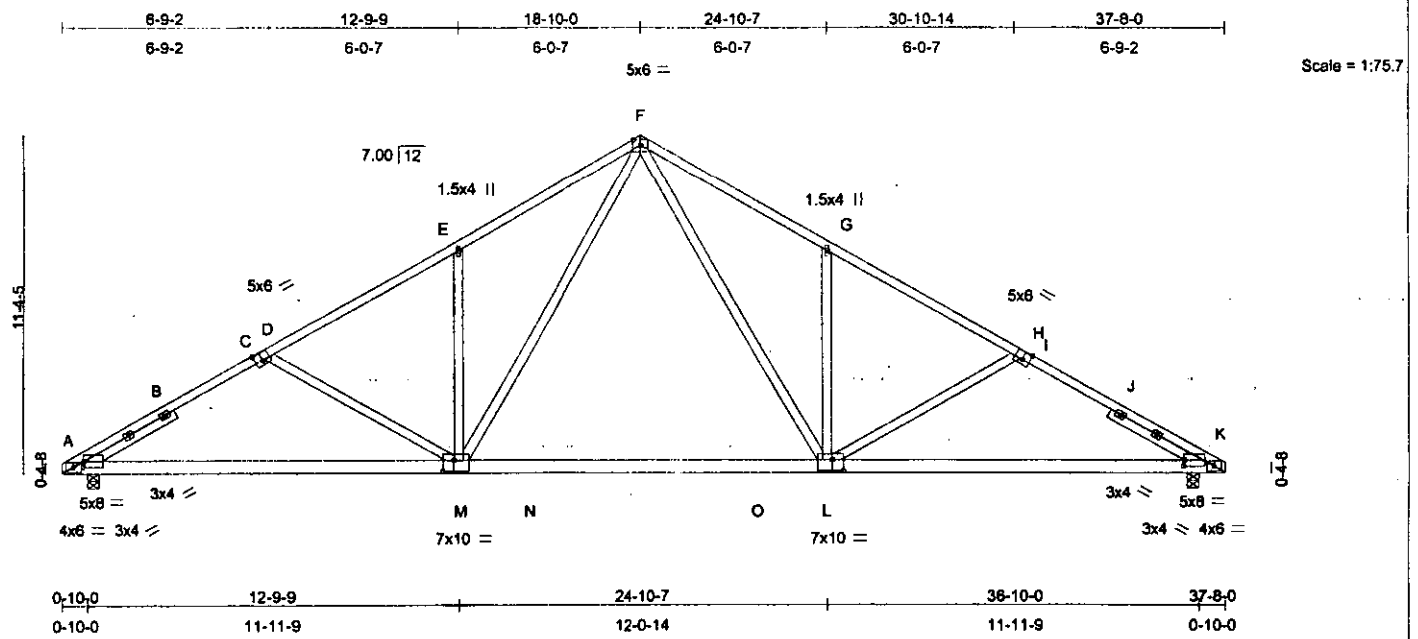


Plate Offsets (X,Y): [A:0-0-8,0-2-8], [C:0-3-0,0-3-4], [I:0-3-0,0-3-4], [K:0-0-8,0-2-8], [L:0-4-4,0-4-8], [M:0-4-4,0-4-8]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/def L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plates Increase 1.15	TC 0.44	Vert(LL) -0.42 L-M >999 480	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.89	Vert(TL) -0.61 L-M >733 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.93	Horz(TL) 0.10 K n/a n/a		
BCDL 10.0	Code IRC2003/TPI2002	(Matrix)			Weight: 241 lb

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 6 SYP No.2  
WEBS 2 X 4 SYP No.3  
SLIDER Left 2 X 4 SYP No.3 3-4-15, Right 2 X 4 SYP No.3 3-4-15

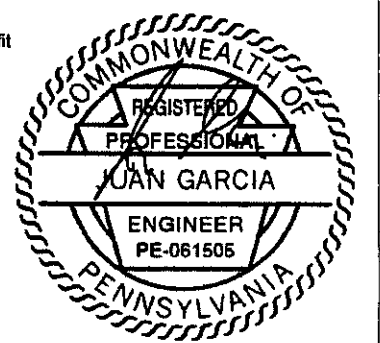
**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 3-1-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) A=2016/0-4-8, K=2016/0-4-8  
Max Horz A=-335(load case 4)  
Max Uplift A=-306(load case 6), K=-306(load case 7)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD A-B=-3424/595, B-C=-3310/624, C-D=-3188/625, D-E=-2958/542, E-F=-2950/700, F-G=-2950/700, G-H=-2958/542,  
H-I=-3188/625, I-J=-3310/624, J-K=-3424/595  
BOT CHORD A-M=-508/2864, M-N=-122/1797, N-O=-122/1797, L-O=-122/1797, K-L=-446/2864  
WEBS E-M=-503/277, G-L=-503/277, D-M=-493/262, F-M=-337/1377, F-L=-337/1377, H-L=-493/262

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 90mph; h=22ft; TCCL=5.0psf; BCCL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  - One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at j(s) A and K.
  - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



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**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.**  
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D38-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



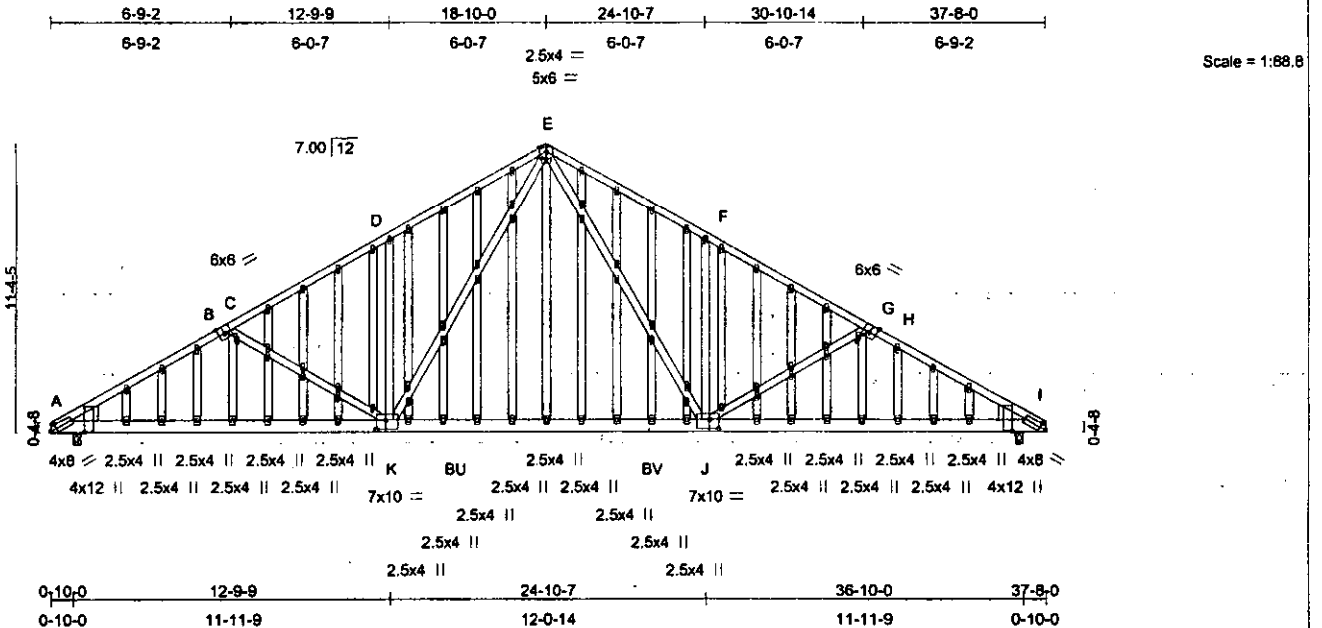


Plate Offsets (X,Y): [A:0-0-3,1-2-15], [A:0-2-12,0-2-0], [B:0-3-0,Edge], [C:0-1-15,0-0-8], [E:0-2-0,0-0-8], [G:0-1-15,0-0-8], [H:0-3-0,Edge], [I:0-0-3,1-2-15], [I:0-2-12,0-2-0], [J:0-4-4,0-4-8], [K:0-4-4,0-4-8]

<b>LOADING (psf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plates Increase 1.15	TC 0.95	Vert(LL) -0.42 J-K >999 480	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.89	Vert(TL) -0.60 J-K >749 360		
BCLL 0.0	Rep Stress Incr NO	WB 0.93	Horz(TL) 0.10 I n/a n/a		
BCDL 10.0	Code IRC2003/TPI2002	(Matrix)			Weight: 448 lb

**LUMBER**  
 TOP CHORD 2 X 4 SYP No.2  
 BOT CHORD 2 X 6 SYP No.1 \*Except\*  
 J-K 2 X 6 SYP No.2  
 WEBS 2 X 4 SYP No.3  
 OTHERS 2 X 4 SYP No.3  
 WEDGE  
 Left: 2 X 8 SYP No.2, Right: 2 X 8 SYP No.2

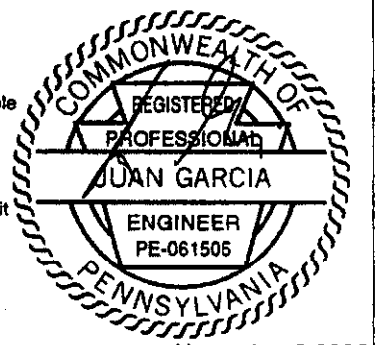
**BRACING**  
 TOP CHORD Structural wood sheathing directly applied or 2-10-14 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS (lb/size)** A=2021/0-3-8, I=2021/0-3-8  
 Max Horz A=335(load case 5)  
 Max Uplift A=-307(load case 6), I=-307(load case 7)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
 TOP CHORD A-B=-3473/631, B-C=-3210/632, C-D=-2977/545, D-E=-2964/700, E-F=-2964/700, F-G=-2977/545, G-H=-3210/632, H-I=-3473/631  
 BOT CHORD A-K=-516/2881, K-BU=-123/1808, BU-BV=-123/1808, J-BV=-123/1808, I-J=-455/2881  
 WEBS D-K=-494/273, F-J=-494/273, C-K=-497/271, E-K=-337/1384, E-J=-337/1384, G-J=-497/271

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 90mph; h=22ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - All plates are 1.5x4 MT20 unless otherwise indicated.
  - Gable studs spaced at 1-4-0 oc.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  - One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at jt(s) A and I.
  - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



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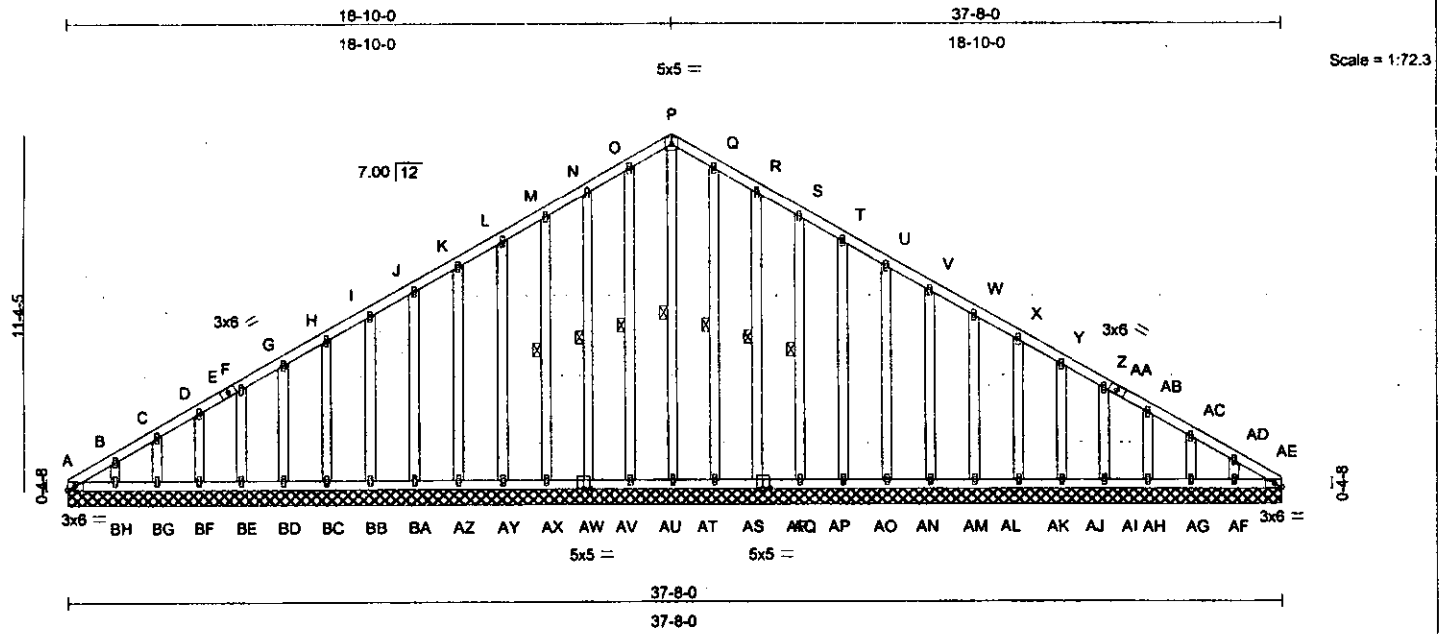


Plate Offsets (X,Y): [AE:0-2-10,Edge], [AQ:0-2-8,0-0-4], [AW:0-2-8,0-0-4]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.13	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plates Increase 1.15	BC 0.04	Vert(TL)	n/a	-	n/a		
BCLL 0.0	Lumber Increase 1.15	WB 0.12	Horz(TL)	0.01	AE	n/a		
BCDL 10.0	Rep Stress Incr NO	(Matrix)						
	Code IRC2003/TPI2002							Weight: 345 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2 X 4 SYP No.3	WEBS 1 Row at midpt P-AT, O-AU, N-AV, M-AX, Q-AS, R-AR, S-AP

**REACTIONS (lb/size)** A=69/37-8-0, AT=173/37-8-0, AU=185/37-8-0, AV=181/37-8-0, AX=186/37-8-0, AY=187/37-8-0, AZ=187/37-8-0, BA=187/37-8-0, BB=186/37-8-0, BC=190/37-8-0, BD=160/37-8-0, BE=129/37-8-0, BF=138/37-8-0, BG=125/37-8-0, BH=163/37-8-0, AS=185/37-8-0, AR=181/37-8-0, AE=69/37-8-0, AP=188/37-8-0, AO=187/37-8-0, AN=187/37-8-0, AM=187/37-8-0, AL=186/37-8-0, AK=190/37-8-0, AJ=160/37-8-0, AI=129/37-8-0, AH=136/37-8-0, AG=125/37-8-0, AF=163/37-8-0

Max Horz A=-337(load case 4)  
 Max Uplift A=98(load case 4), AU=-18(load case 5), AV=-56(load case 6), AX=-47(load case 6), AY=-45(load case 6), AZ=-48(load case 6), BA=-46(load case 6), BB=-46(load case 6), BC=-48(load case 6), BD=-46(load case 6), BE=-46(load case 6), BF=-46(load case 6), BG=-43(load case 6), BH=-65(load case 6), AR=-58(load case 7), AE=23(load case 5), AP=-47(load case 7), AO=-45(load case 7), AN=-46(load case 7), AM=-46(load case 7), AL=-46(load case 7), AK=-46(load case 7), AJ=-46(load case 7), AI=-46(load case 7), AH=-46(load case 7), AG=-43(load case 7), AF=-64(load case 7)

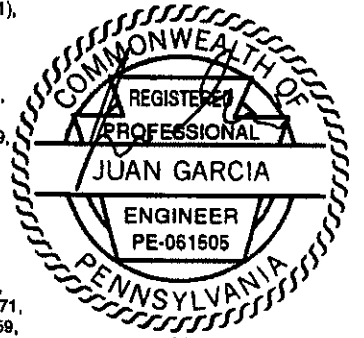
Max Grav A=181(load case 5), AT=196(load case 7), AU=186(load case 10), AV=182(load case 10), AX=186(load case 1), AY=187(load case 1), AZ=187(load case 10), BA=187(load case 10), BB=186(load case 1), BC=190(load case 1), BD=160(load case 10), BE=129(load case 1), BF=138(load case 1), BG=125(load case 10), BH=163(load case 10), AS=186(load case 11), AR=182(load case 7), AE=93(load case 7), AP=188(load case 1), AO=187(load case 1), AN=187(load case 11), AM=187(load case 11), AL=188(load case 1), AK=190(load case 1), AJ=160(load case 11), AI=129(load case 1), AH=136(load case 1), AG=125(load case 11), AF=163(load case 11)

**FORCES (lb) - Maximum Compression/Maximum Tension**

**TOP CHORD** A-B=314/211, B-C=282/203, C-D=257/200, D-E=-233/194, E-F=-227/197, F-G=-208/194, G-H=-184/191, H-I=-160/188, I-J=-135/185, J-K=-111/182, K-L=-86/194, L-M=-62/219, M-N=-44/245, N-O=-44/275, O-P=-43/276, P-Q=43/276, Q-R=44/266, R-S=44/227, S-T=44/192, T-U=44/157, U-V=44/123, V-W=44/89, W-X=44/63, X-Y=49/56, Y-Z=74/59, Z-AA=92/63, AA-AB=98/59, AB-AC=-122/66, AC-AD=-154/88, AD-AE=-199/75

**BOT CHORD** A-BH=-61/185, BG-BH=-61/185, BF-BG=-61/185, BE-BF=-61/185, BD-BE=-61/185, BC-BD=-61/185, BB-BC=-61/185, BA-BB=-61/185, AZ-BA=-61/185, AY-AZ=-61/185, AX-AY=-61/185, AW-AX=-61/185, AV-AW=-61/185, AU-AV=-61/185, AT-AU=-61/185, AS-AT=-61/185, AR-AS=-61/185, AQ-AR=-61/185, AP-AQ=-61/185, AO-AP=-61/185, AN-AO=-61/185, AM-AN=-61/185, AL-AM=-61/185, AK-AL=-61/185, AJ-AK=-61/185, AI-AJ=-61/185, AH-AI=-61/185, AG-AH=-61/185, AF-AG=-61/185, AE-AF=-61/185

**WEBS** P-AT=-183/0, O-AU=-106/31, N-AV=-109/69, M-AX=-107/60, L-AY=-107/59, K-AZ=-107/59, J-BA=-107/59, I-BB=-107/59, H-BC=-107/59, G-BD=-107/59, F-BE=-107/59, D-BF=-107/59, C-BG=-103/58, B-BH=-122/73, Q-AS=-106/9, R-AR=-109/71, S-AP=-107/60, T-AO=-107/59, U-AN=-107/59, V-AM=-107/59, W-AL=-107/59, X-AK=-107/59, Y-AJ=-107/59, Z-AI=-107/59, AB-AH=-107/59, AC-AG=-103/58, AD-AF=-122/72



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Continued on page 2

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 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



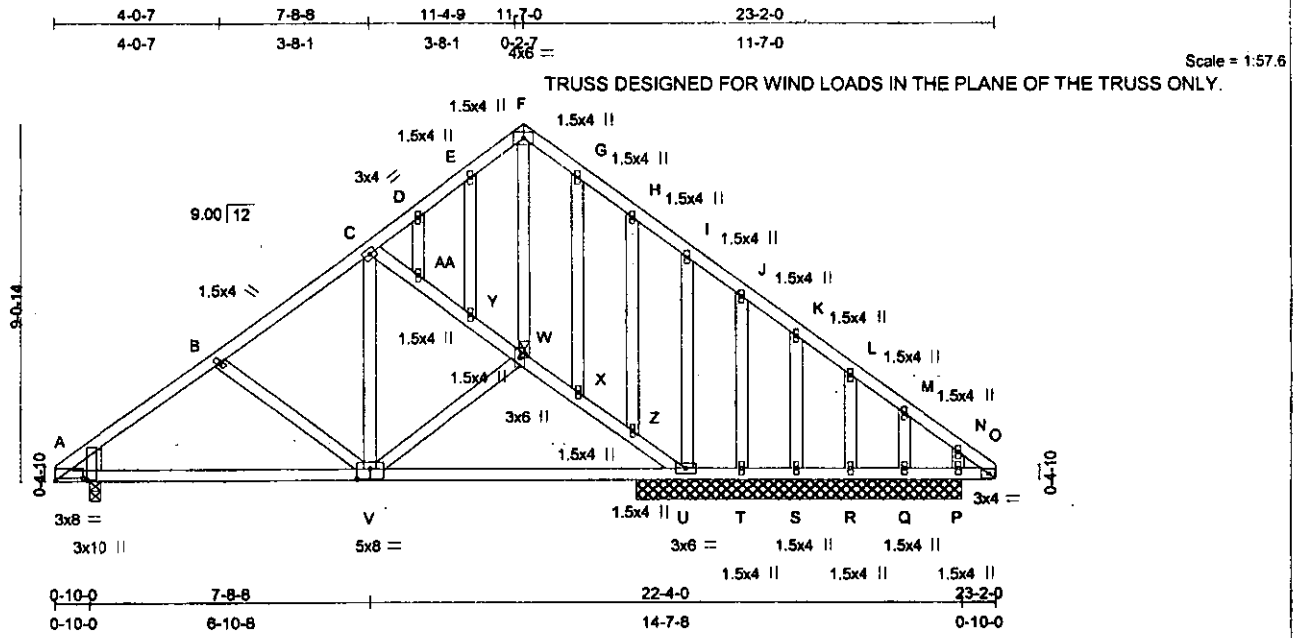


Plate Offsets (X,Y): [A:0-0-4,0-9-4], [A:0-8-3,0-0-14], [V:0-4-0,0-3-0]

<b>LOADING (psf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/def L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plating Increase 1.15	TC 0.65	Vert(LL) -0.06 A-V >999 480	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.44	Vert(TL) -0.15 A-V >999 360		
BCLL 0.0	Rep Stress Incr NO	WB 0.27	Horz(TL) 0.02 P n/a n/a		
BCDL 10.0	Code IRC2003/TPI2002	(Matrix)			Weight: 177 lb

**LUMBER**  
 TOP CHORD 2 X 4 SYP No.2  
 BOT CHORD 2 X 4 SYP No.2  
 WEBS 2 X 4 SYP No.3  
 WEDGE  
 Left: 2 X 8 SYP No.2

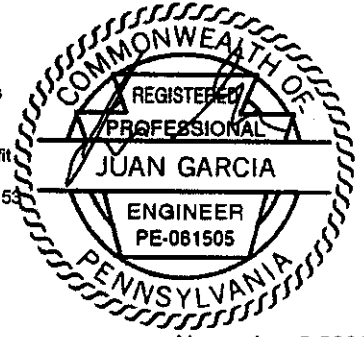
**BRACING**  
 TOP CHORD Structural wood sheathing directly applied or 5-9-11 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 JOINTS 1 Brace at Jt(s): W

**REACTIONS (lb/size)** U=887/8-0-0, T=38/8-0-0, S=186/8-0-0, R=154/8-0-0, Q=9/8-0-0, P=341/8-0-0, A=797/0-3-8  
 Max Horz A=267(load case 5)  
 Max Uplift U=-80(load case 6), T=-130(load case 2), S=-53(load case 7), R=-34(load case 7), Q=-169(load case 7), P=-39(load case 5), A=-122(load case 6)  
 Max Grav U=887(load case 1), T=43(load case 11), S=187(load case 11), R=154(load case 1), Q=70(load case 5), P=341(load case 1), A=797(load case 1)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
 TOP CHORD A-B=-1047/200, B-C=-816/185, C-D=-214/178, D-E=-151/191, E-F=-131/205, F-G=-127/193, G-H=-156/174, H-I=-211/151, I-J=-128/77, J-K=-172/51, K-L=-169/35, L-M=-157/37, M-N=-193/50, N-O=-164/39  
 BOT CHORD A-V=-200/778, U-V=-132/682, T-U=-36/161, S-T=-36/161, R-S=-36/161, Q-R=-36/161, P-Q=-36/161, O-P=-36/161  
 WEBS C-AA=-583/158, Y-AA=-584/161, W-Y=-620/179, W-X=-892/124, X-Z=-721/119, U-Z=-736/121, F-W=-138/19, G-X=-47/18, E-Y=-68/31, C-V=0/412, B-V=-257/169, V-W=-134/14, H-Z=-28/29, I-U=-239/134, J-T=-47/45, K-S=-112/78, L-R=-117/61, M-Q=-72/119, N-P=-183/7, D-AA=-7/10

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-02; 90mph; h=22ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint U, 130 lb uplift at joint T, 53 lb uplift at joint S, 34 lb uplift at joint R, 169 lb uplift at joint Q, 39 lb uplift at joint P and 122 lb uplift at joint A.
  - 6) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



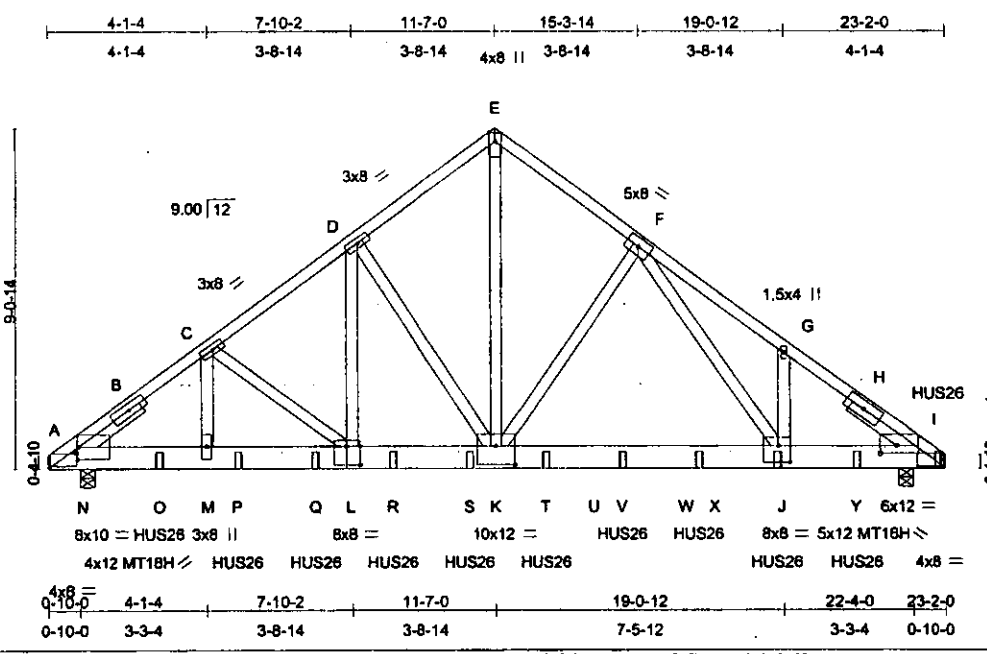


Plate Offsets (X,Y): [A:0-0-5,0-4-0], [A:0-0-11,0-2-2], [I:0-5-1,0-2-8], [I:1-2-8,Edge], [J:0-3-8,0-5-4], [K:0-8-0,0-6-0], [L:0-4-0,0-6-0]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plates Increase 1.15	TC 0.67	Vert(LL) -0.22 J-K >999 480	MT20	244/190
TCDL 10.0	Lumber Increase 1.15	BC 0.94	Vert(TL) -0.40 J-K >680 360	MT18H	244/190
BCLL 0.0	Rep Stress Incr NO	WB 0.86	Horz(TL) 0.07 l n/a n/a		
BCDL 10.0	Code IRC2003/TPI2002	(Matrix)			Weight: 551 lb

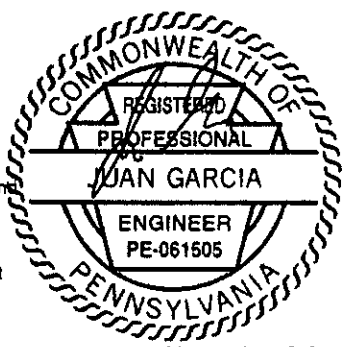
**LUMBER**  
 TOP CHORD 2 X 4 SYP No.2  
 BOT CHORD 2 X 8 SYP No.2 \*Except\*  
 I-L 2 X 8 SYP 2400F 2.0E  
 WEBS 2 X 4 SYP No.3 \*Except\*  
 E-K 2 X 4 SYP No.2  
 SLIDER Left 2 X 4 SYP No.3 1-11-13, Right 2 X 4 SYP No.3 1-11-13

**BRACING**  
 TOP CHORD Structural wood sheathing directly applied or 4-10-12 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) A=11080/0-4-8, I=13025/0-4-8  
 Max Horz A=-262(load case 3)  
 Max Uplift A=-1803(load case 5), I=-2050(load case 6)

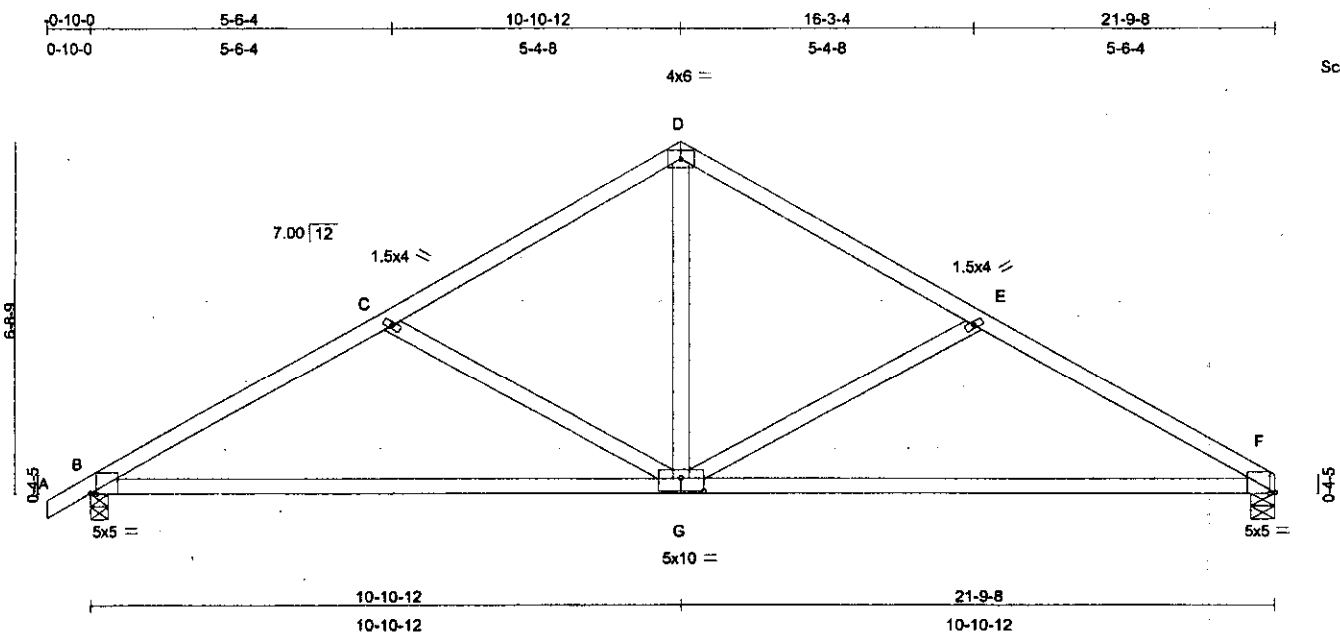
**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD A-B=-18935/2654, B-C=-18819/2670, C-D=-13755/2212, D-E=-11112/1859, E-F=-11147/1882, F-G=-17679/2889, G-H=-17668/2768, H-I=-17822/2755  
 BOT CHORD A-N=-2168/13455, N-O=-2168/13455, M-O=-2168/13455, M-P=-2168/13455, P-Q=-2168/13455, L-Q=-2168/13455, L-R=-1887/10945, R-S=-1887/10945, K-S=-1887/10945, K-T=-1544/10326, T-U=-1544/10326, U-V=-1544/10326, V-W=-1544/10326, W-X=-1544/10326, J-X=-1544/10326, J-Y=-2137/14122, I-Y=-2137/14122  
 WEBS C-M=-545/3817, C-L=-3213/615, D-L=-736/4554, D-K=-3846/744, E-K=-2106/12886, F-K=-2708/577, F-J=-1179/7001, G-J=-83/164

- NOTES**
- 3-ply truss to be connected together with 10d Common(.148"x3") Nails as follows:  
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.  
 Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.  
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
  - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
  - Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 90mph; h=22ft; TCCL=5.0psf; BCCL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - All plates are MT20 plates unless otherwise indicated.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1803 lb uplift at joint A and 2050 lb uplift at joint I.
  - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



November 2, 2006





Scale = 1:43.0

Plate Offsets (X,Y): [B:0-1-3,Edge], [F:0-1-3,Edge], [G:0-5-0,0-3-0]

<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	2-0-0	TC 0.51	in (loc) l/defi L/d	MT20	244/190
TCDL 10.0	Plates Increase 1.15	BC 0.75	Vert(LL) -0.21 B-G >999 480		
BCLL 0.0	Lumber Increase 1.15	WB 0.34	Vert(TL) -0.56 B-G >462 360		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.05 F n/a n/a		
	Code IRC2003/TPI2002				Weight: 99 lb

**LUMBER**  
 TOP CHORD 2 X 4 SYP No.2  
 BOT CHORD 2 X 4 SYP No.2  
 WEBS 2 X 4 SYP No.3

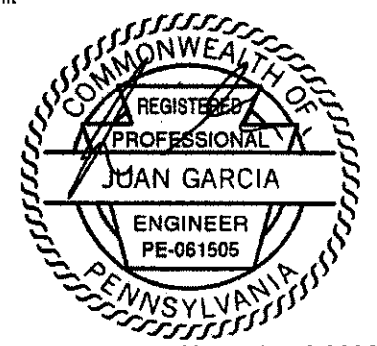
**BRACING**  
 TOP CHORD Structural wood sheathing directly applied or 4-2-8 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS (lb/size)** F=1068/0-5-8, B=1152/0-4-0  
 Max Horz B=204(load case 5)  
 Max Uplift F=-174(load case 7), B=-223(load case 6)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
 TOP CHORD A-B=0/32, B-C=-1638/344, C-D=-1226/271, D-E=-1226/271, E-F=-1634/345  
 BOT CHORD B-G=-255/1344, F-G=-221/1340  
 WEBS C-G=-475/231, D-G=-98/692, E-G=-471/234

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 90mph; h=22ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  - One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at jt(s) F and B.
  - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

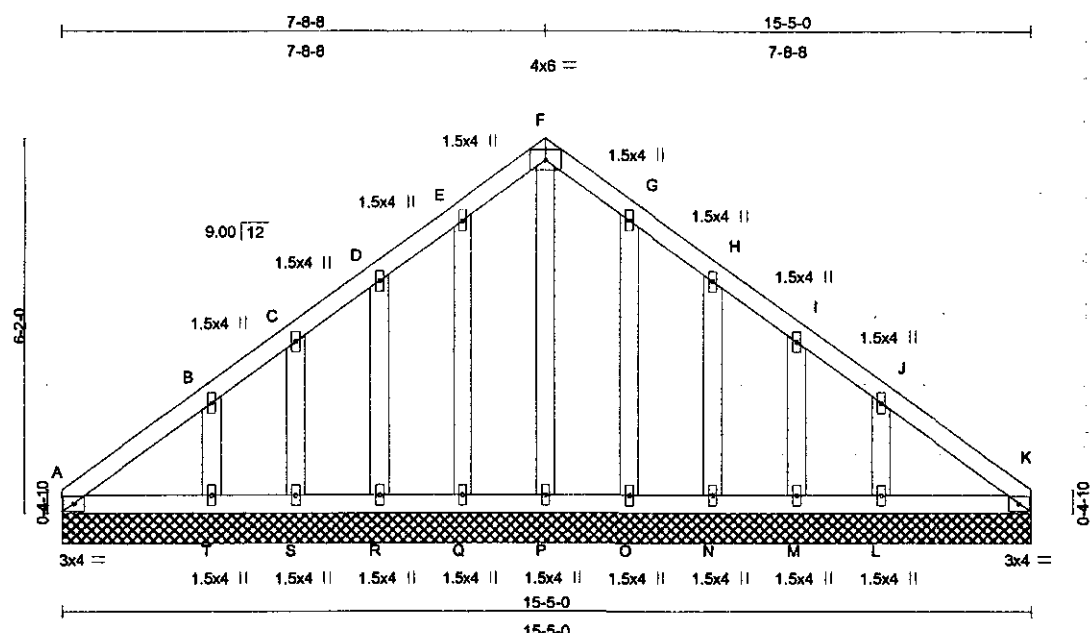
**LOAD CASE(S)** Standard



November 2, 2006

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.**  
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





Scale = 1:37.2

<b>LOADING (psf)</b> TCLL 30.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	<b>SPACING</b> 2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr NO Code IRC2003/TPI2002	<b>CSI</b> TC 0.08 BC 0.05 WB 0.06 (Matrix)	<b>DEFL</b> in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(TL) n/a - n/a 999 Horz(TL) 0.00 K n/a n/a	<b>PLATES</b> MT20 <b>GRIP</b> 244/190  Weight: 98 lb
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<b>LUMBER</b> TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3	<b>BRACING</b> TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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**REACTIONS (lb/size)** A=111/15-5-0, K=111/15-5-0, P=161/15-5-0, Q=187/15-5-0, R=169/15-5-0, S=90/15-5-0, T=240/15-5-0, O=187/15-5-0, N=169/15-5-0, M=90/15-5-0, L=240/15-5-0  
 Max Horz A=-178(load case 4)  
 Max Uplift A=39(load case 4), K=-2(load case 5), Q=-40(load case 6), R=-67(load case 6), S=-40(load case 6), T=-107(load case 6), O=-37(load case 7), N=-68(load case 7), M=-40(load case 7), L=-107(load case 7)  
 Max Grav A=111(load case 1), K=111(load case 1), P=161(load case 1), Q=190(load case 10), R=169(load case 10), S=90(load case 1), T=240(load case 10), O=190(load case 11), N=169(load case 11), M=90(load case 1), L=240(load case 11)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
**TOP CHORD** A-B=-157/108, B-C=-100/94, C-D=-71/92, D-E=-55/110, E-F=-54/134, F-G=-54/134, G-H=-55/98, H-I=-52/47, I-J=-66/35, J-K=-102/48  
**BOT CHORD** A-T=-35/112, S-T=-35/112, R-S=-35/112, Q-R=-35/112, P-Q=-35/112, O-P=-35/112, N-O=-35/112, M-N=-35/112, L-M=-35/112, K-L=-35/112  
**WEBS** F-P=-99/0, E-Q=-108/54, D-R=-111/79, C-S=-84/57, B-T=-173/114, G-O=-108/50, H-N=-111/80, I-M=-84/57, J-L=-173/114

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 90mph; h=22ft; TCCL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - Gable requires continuous bottom chord bearing.
  - Gable studs spaced at 1-4-0 oc.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint A, 2 lb uplift at joint K, 40 lb uplift at joint Q, 67 lb uplift at joint R, 40 lb uplift at joint S, 107 lb uplift at joint T, 37 lb uplift at joint O, 68 lb uplift at joint N, 40 lb uplift at joint M and 107 lb uplift at joint L.
  - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



November 2, 2006

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